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original to, Permits

TN Dept. of Env. & Conservation

AUG 0 5 2020

Division of Water Resources

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July 30, 2020

Ms. Jessica Murphy
Manager, Compliance and Enforcement Unit
Division of Water Resources
TDEC
William R. Snodgrass – Tennessee Tower
312 Rosa L. Parks Avenue, 11th Floor
Nashville, TN 37243-1102

RE: SOP Application
Southeastern Provision LLC
Bean Station, Grainger County, TN 37708
WPC18-0030

Dear Ms. Murphy:

Per recent written and email correspondence from TDEC and in accordance with the revised CAP submitted in late June 2020 attached please find an application for a new SOP for construction of a new wastewater treatment system and drip dispersal system at Southeastern Provision. The new system will be designed to handle a future flow of up to 100,000 gallons per day and will consist of the following components:

- Anoxic Pretreatment Lagoon
- Cyclic Reactor Lagoon
- Aerobic Sludge Digestion Lagoon
- Effluent Storage Lagoon
- Drip Dispersal System

We are prepared to immediately begin the preparation of detailed plans and specifications upon receipt of authorization from TDEC.

Please advise if additional information is needed at this time.

Very truly yours,

Vernon D. Rowe, P.E.



Tennessee Department of Environment and Conservation Division of Water Resources William R. Snodgrass - Tennessee Tower 312 Rosa L. Parks Avenue, 11th Floor Nashville, Tennessee 37243-1102 (615) 532-0625

APPLICATION FOR A STATE OPERATION PERMIT (SOP)

	Type of application:	New Permit	Permit Reissuance	Permit Mod	lification
Permittee Identification: (Name of city, town, industry, corporation, individual, etc., applying, according to the provisions of Tennessee Code Annotated Section 69-3-108 and Regulations of the Tennessee Water Quality Control Board.)					
Permittee So Name (applicant):	utheastern Provision	on LLC			
Permittee 161 Address:	7 Helton Road, B	ean Station, TN 3	7708	.99	8
Official Carata			1 - · · · ·		
Official Contac	rt: Randy Hodge		Title or Position: W	/astewater M	anager
Mailing Addre	ss: 1617 Helton R	oad	City: Bean Station	State: TI	Zip: 37708
Phone numbe	r(s): (865) 767-23	00	E-mail: southeaste	rnprovision@	gyahoo.com
Optional Conta	act: William J. Gilo	ger	Title or Position:	hief Operatin	g Officer
Address: 1617	Helton Road		City: Bean Station	State: Th	Zip: 37708
Phone numbe	r(s): (482) 350-112	7	E-mail: wgilger@gm	nail.com	
Application Certification (must be signed in accordance with the requirements of Rule 0400-40-0505)					
I certify under penalty of law that this document and all attachments were prepared under my direction or supervision in accordance with a system designed to assure that qualified personnel properly gathered and evaluated the information submitted. Based on my inquiry of the person or persons who manage the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate, and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations. As specified in Tennessee Code Annotated Section 39-16-702(a)(4), this declaration is made under penalty of perjury.					
	e; print or type		Signature		Date, /
	Gilger, Chief Opera	ting Officer	Thin		7/30/2020
CN 1251 (Rev. 03-19) (continued) RDA 23		RDA 2366			

Permit Number: SOP-_____

Facility Identificat	tion:		Existing Permit No.
Facility Name: Southeaster	n Provision LLC		County: Grainger
Facility 1617 H Address or	elton Road, Bean Station	, TN 37708	Latitude: 36 deg 27 min 17 sec N
Location:			Longitude: 83 deg 23 min 47 sec W
Name and distance	to nearest receiving wate	rs: Honey Creek Tributary to Cher	okee Reservoir; 300 ft
If any other State o numbers:	r Federal Water/Wastewat	er Permits have been obtained f	or this site, list their permit
GRA0000013 (exp	ired 5/11/2017); SOP 180	010 (Pump and Haul)	
Name of company	or governmental entity tha	at will operate the permitted syst	em; Southeastem Provision LLC
Operator address:	1617 Helton Road, Bean	Station. TN 37708	
Has the owner/ope with the Tennessee application treatme	rator filed for a Certificate Regulatory Authority (TRA ent systems)?	of Convenience & Necessity (CCI A) (may be required for collection No M N/A	systems and land
explain how and where the renewal terms of the notapplicable Complete the follow	nen the ownership will be ne contract for operations.	the facility/site or if the applican transferred or describe the contr the entity type, number of design	actual arrangement and
wastewater flow: Entity Type	Number of	Design Units	Flow (gpd)
City, town or county	No. of connections:	Design Offics	Flow (gpa)
Subdivision	No. of homes:	Avg. No. bedrooms per home:	
School	No. of students:	Size of cafeteria(s): No. of showers:	
Apartment	No. of units:	No. units with Washer/Dryer hook No. units without W/D hookups:	kups:
Commercial Business	No. of employees:	Type of business:	
☐ Industry	No. of employees: 150	Product(s) manufactured: Beef	100,000
Resort	No. of units:		
Camp	No. of hookups:		
RV Park	No. of hookups:	No. of dump stations:	
Car Wash	No. of bays:		
Other			
Wastewater is generate	ed from the processing of beef cat	result in wastewater generation. ttle and sanitation of the processing facility ion shifts. The plant operates 5 to 6 days	. Wastewater is generated per week.

CN 1251 (Rev. 03-19) RDA 2366

Permit Number: SOP-____

Engineering Report (required for collection systems and/or land application					
treatment systems):					
Prepared in accordance with Rule 0400-40-0503 and Section 1.2 of the State of Tennesse Design Criteria for Sewage Works	:e				
Attached, or					
	lo				
Operation and Maintenance Inspection Schedule Submitted:					
Approved? Yes. Date: N	lo				
Wastewater Collection System:					
Wastewater Collection System:					
System type (i.e., gravity, low pressure, vacuum, combination, etc.):					
System Description:					
Describe methods to prevent and respond to any bypass of treatment or discharges (i.e., pow failures, equipment failures, heavy rains, etc.):	<i>i</i> er				
In the event of a system failure describe means of operator notification:					
List the emergency contact(s) (name/phone):					
For low-pressure systems, who is responsible for maintenance of STEP/STEG tanks and pumps or grinder pumps (list all contact information)?					
Approximate length of sewer (excluding private service lateral):					
Number/hp of lift stations: / Number/hp of lift pumps /					
Number/volume of low pressure and or grinder pump tanks / Number/volume septic tanks /					
Attach a schematic of the collection system. Attached					
If this is a satellite sewer and you are tying in to another sewer system complete the following section, listing tie-in points to the sewer system and their location (attach additional sheets as necessary):					
<u>Tie-in Point</u> <u>Latitude (xx.xxxx°)</u> <u>Longitude (xx.xxxx°)</u>					

Permit Number: SOP-_____

Land Application Treatment System:				
Type of Land Application Treatment System: 🗸 Drip 🔃 Spray 🗸 Other, explain:				
Type of treatment facility preceding land application (recirculating media filters, lagoons, other, etc.): Screening, flow equalization, dissolved air flotation, anoxic pretreatment lagoon, oxic/anoxic biological treatment lagoon, storage lagoon, filtration				
Attach a treatment schematic. 🗸 Attached				
Describe methods to prevent and respond to any bypass of treatment or discharges (i.e., power failures, equipment failures, heavy rains, etc.): In the event of a power failure flow from the processing plant will stop. In the event of equipment failures, requipment failures, heavy rains, etc.):				
For New or Modified Projects:				
Name of Developer for the project: Southeastern Provision LLC				
Developer address and phone number: 1617 Helton Road, Bean Station, TN 37708; (865) 767-2300				
For land application, list: Proposed acreage involved: 10.1 to 11.5 acres *see note below Inches/week gpd/sq.ft loading rate to be applied: 0.2 gpd/sqft				
Is wastewater disinfection proposed?				
Yes Describe land application area access: FENCED WITH LOCKED GATE				
No Describe how access to the land application area will be restricted:				
Attach required additional Engineering Report Information (see <u>website</u> for more				
information)				
✓ Topographic map (1:24,000 scale presented at a six inch by six inch minimum size) showing				
the location of the project including quadrangle(s) name(s) GPS coordinates, and latitude and longitude in decimal degrees should also be included.				
Scaled layout of facility showing the following: lots, buildings, etc. being served, the				
wastewater collection system routes, the pretreatment system location, the proposed land				
application area(s), roads, property boundaries, and sensitive areas such as streams, lakes,				
springs, wells, wellhead protection areas, sinkholes and wetlands.				
Soils information for the proposed land disposal area in the form of a Water Resources Soils				
Map per Chapter 16 and 17 State of Tennessee Design Criteria for Sewage Work. The soils				
information should include soil depth (borings to a minimum of 4 feet or refusal) and soil				
profile description for each soil mapped.				
✓ Topographic map of the area where the wastewater is to be land applied with no greater				
than ten foot contours presented at a minimum size of 24 inches by 24 inches.				
Describe alternative application methods based on the following priority rating: (1)				
connection to a municipal/public sewer system, (2) connection to a conventional subsurface				
disposal system as regulated by the Division of Groundwater Protection, and/or (3) land application.				

(*) The facility currently has subsurface disposal areas consisting of 4,000 ft of LPP and 11,000 ft of chambers that were originally permitted and installed under expired Permit No. GRA0000013. An additional 5,000 feet of chamber was permitted but not installed. TDEC has expressed concern with continued use of the existing areas due to high hydraulic and organic loadings in the past. Southeastern Provision plans to work with TDEC to evaluate the existing subsurface disposal areas to determine if they can possibly be used in the future. If the existing subsurface disposal areas are determined to be usable they will be able to handle up to 12,000 gpd at a loading of 0.2 gal per day per sf. This will reduce the requirement for drip irrigation to 10.1 acres. If the existing subsurface areas can not be used in the future a drip system of up to 11.5 acres will have to be developed.

Permit Number: SOP-_____

For Drip Dispersal Systems Only: Unless otherwise determined by the Department, sewage treatment effluent wells, i.e, large capacity treatment/drip dispersal systems after approval of the SOP Application, will be issued an UIC tracking number and will be authorized as Permit by Rule per UIC Rule 0400-45-0614(2) and upon issue of a State Operating Permit and Sewage System Construction Approval by the Department. Describe the following:	□ N/A			
The area of review (AOR) for each Drip Dispersal System shall, unless otherwise s	pecified by the			
Department, consist of the area lying within a one mile radius or an area defined by using calculations under 0400-45-0609 of the Drip Dispersal System site or facility, and shall include, but not be limited to general surface geographic features, general subsurface geology, and general demographic and cultural features within the area. Attach to this part of the application a general characterization of the AOR, including the following: (This can be in narrative form)				
A general description of all past and present groundwater uses as well as the general a flow direction and general water quality.				
A general description of the population and cultural development within the AOR (i.e. a commercial, residential or mixed)				
Mature of injected fluid to include physical, chemical, biological or radiological characters	eristics.			
If groundwater is used for drinking water within the area of review, then identify and locate on a topographic map all groundwater withdrawal points within the AOR, which supply public or private drinking water systems. Or supply map showing general location of publicly supplied water for the area (this can be obtained from the water provider)				
If the proposed system is located within a wellhead protection area or source water protection area designated by Rule 0400-45-0134, show the boundary of the protection area on the facility site plan.				
Description of system, Volume of injected fluid in gallons per day based upon design flow, including any monitoring wells				
✓ Nature and type of system, including installed dimensions of wells and construction materials				
Pump and Haul:	✓ N/A			
Reason system cannot be served by public sewer:				
Distance to the nearest manhole where public sewer service is available:				
When sewer service will be available:				
Volume of holding tank: gal.				
Tennessee licensed septage hauler (attach copy of agreement):				
Facility accepting the septage (attach copy of acceptance letter):				
Latitude and Longitude (in decimal degrees) of approved manhole for discharge of septage				
Describe methods to prevent and respond to any bypass of treatment or discharges (i.e., power failures, equipment failures, heavy rains, etc.):				

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Holding Ponds (for non-domestic wastewater only):	✓ N/A			
Pond use: Recirculation Sedimentation Cooling Other (describe):				
Describe pond use and operation:				
If the pond(s) are existing pond(s), what was the previous use?				
Have you prepared a plan to dispose of rainfall in excess of evaporation? Yes	☐ No			
If so, describe disposal plan:				
Is the pond ever dewatered? Yes No				
If so, describe the purpose for dewatering and procedures for disposal of wastewater and/or sludge:				
Is(are) the pond(s) aerated? Yes No				
Volume of pond(s): gal. Dimensions:				
Is the pond lined (Note if this is a new pond system it must be lined for SOP coverage.				
Otherwise, you must apply for an Underground Injection Control permit.)? 🔲 Yes 🔲 No				
Describe the liner material (if soil liner is used give the compaction specifications):				
Is there an emergency overflow structure? Yes No				
If so, provide a design drawing of structure.				
Are monitoring wells or lysimeters installed near or around the pond(s)?	No			
If so, provide location information and describe monitoring protocols (attach additional sheets as necessary):				

Permit Number: SOP-_____

Mobile Wash Operations:	☑ N/A			
Individual Operator	Fleet Operation Operator			
Indicate the type of equipment, vehicle, o	r structure to be washed during normal			
operations (check all that apply):	_			
Cars	Parking Lot(s): sq. ft.			
Trucks	Windows: sq. ft.			
Trailers (Interior washing of dump-trailers	Structures (describe):			
or tanks, is prohibited.)				
Other (describe):				
Wash operations take place at (check all t				
Car sales lot(s)	Public parking lot(s)			
Private industry lot(s)	Private property(ies)			
County(ies), list:	Statewide			
Wash equipment description:	T			
Truck mounted	Trailer mounted			
Rinse tank size(s) (gal.):	Mixed tanks size(s) (gal.):			
Collection tank size(s) (gal.): Pressure washer:	Number of tanks per vehicle:			
	psi (rated) gpm (rated) ectric			
Vacuum system manufacturer/model:	Vacuum system capacity: inches Hg			
Describe any other method or system used to contain and collect wastewater:				
List the public sewer system where you are p	ermitted or have written permission to dischar	ge		
waste wash water (include a copy of the pe	ermit or permission letter):			
Are chemicals pre-mixed, prior to arriving at wash location? Yes No				
Describe all soaps, detergents, or other chemicals used in the wash operation (attach				
additional sheets as necessary):				
Chemical name: Manu	ufacturer: Primary CAS No. or Product	t No.		
	N			

PRELIMINARY ENGINEERING REPORT

Preliminary Engineering Report Proposed Process Wastewater Management Syst Southeastern Provision LLC Bean Station, Tennessee

PURPOSE AND NEED FOR THE PROPOSED PROJECT

Southeastern Provision LLC (SP) operates a beef processing plant in Bean Station, Tennessee. The existing wastewater management system for the facility consists of a physical/chemical pretreatment system followed by a subsurface disposal system (low pressure pipe and chambers). The existing system is incapable of adequately treating and disposing of the process wastewater generated by the processing plant. As a result of the inadequacy of the existing system the Tennessee Department of Environment and Conservation (TDEC) has ordered SP to cease using the existing subsurface disposal system and issued a temporary State Operating Permit (SOP No. 18010) to pump and haul process wastewater offsite to a permitted facility for handling. Additionally, TDEC has issued a Consent Order requiring SP to develop a Corrective Action Plan (CAP) to apply for a new SOP to allow upgrading of the wastewater treatment system and subsurface disposal system to comply with TDEC requirements. The proposed project is needed to comply with TDEC directives and assure the long term sustainability of the SP processing plant.

DESCRIPTION OF EXISTING SYSTEM

Figure No. 1 provides a flow schematic of the existing wastewater treatment system and subsurface disposal system. The system consists of the following components:

- Primary Screen
- Transfer Pumps
- Secondary Screen
- Aerated Equalization Basin
- Dissolved Air Flotation (DAF) Feed Pumps
- Floc Tube
- Dissolved Air Flotation (DAF) unit with Primary Coagulant (metal salt) and Polymer (anionic) Feed Systems
- Effluent Storage Tanks
- Subsurface Disposal System Dosing Pumps
- Sock Filters
- Subsurface Disposal System
- DAF Skimmings Storage Tank
- DAF Skimmings Pumps

Table No. 1 provides a summary of the existing treatment components and treatment capacities.

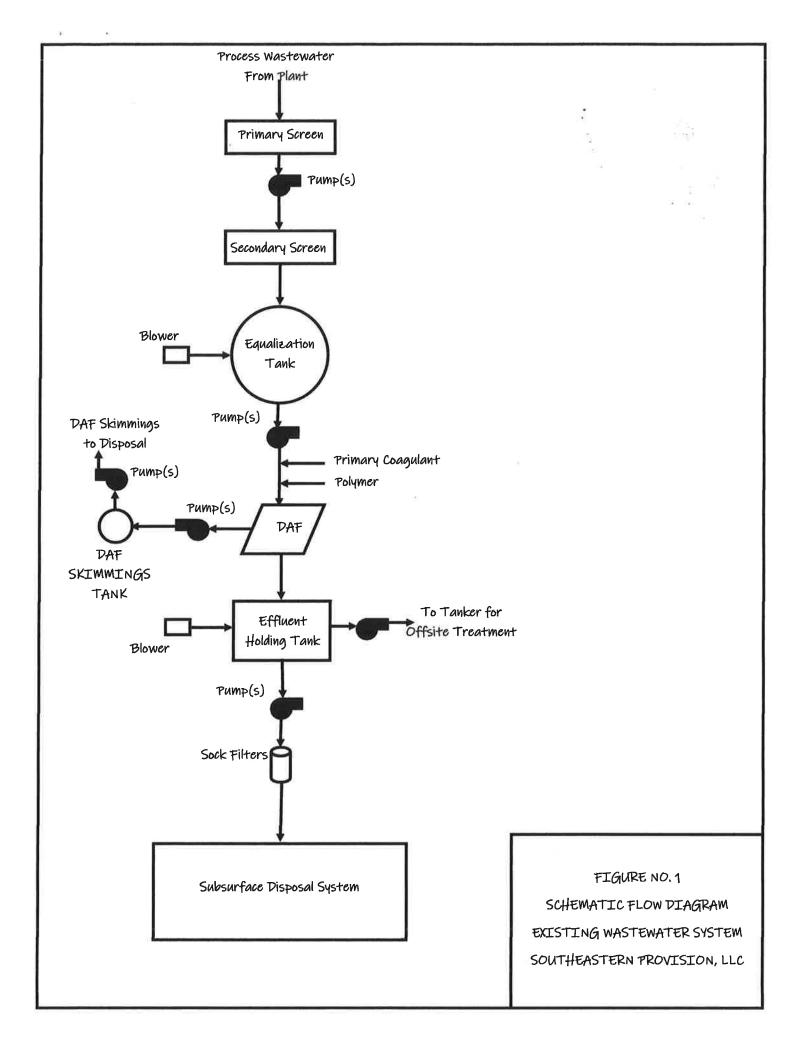


Table No. 1
Existing Wastewater Treatment System Components

Component	Description	Capacity
Primary Screen	24" by 72" Hycor Rotary Drum Screen	400 gpm
Transfer Pumps	Two Summit Self Priming Centrifugal Pumps; 3 by 3; 15 hp; 1800 rpm	250 gpm
Secondary Screen	72" Parabolic Screen	600 gpm
Equalization Tank	70,000 gallon Circular Concrete Basin with diffused aeration; dual 5 hp PD Blowers	16.8 hours detention time at peak flow of 100,000 gpd
DAF Feed Pumps	Two Durco Centrifugal Pumps; 3 by 2; 15 hp; 3600 rpm; VFD's	250 gpm
DAF Floc Tube	4" Diam; 75 ft long	250 gpm
DAF Unit	Stork; 13' by 6' by 6'; recycle pressurization; 3' by 6' sludge hopper	250 gpm
Effluent Storage Tanks	50,000 gallon Multi-compartment concrete tank with diffused aeration	12 hours detention time at peak flow of 100,000 gpd
Subsurface Disposal System Dosing Pumps	Two Durco Centrigufal; 3 x 2; 15 hp; 3600 rpm	250 gpm
Sock Filters	Pall Filters; Dual 50 micron plus Dual 25 micron in series	250 gpm
Subsurface Disposal System (currently out of service due to excessive hydraulic and organic loadings)	4,000 ft of Low Pressure Pipe; 11,000 ft of Chamber installed; 5,000 ft of chamber approved but not installed	The subsurface disposal system hydraulic loading rate is limited to 0.2 gpd/sf to assure Nitrate Nitrogen requirements are met. Trenches are 3 ft wide resulting in a total application area of 60,000 sf. Hydraulic loading capacity is limited to 12,000 gpd.
DAF Skimmings Storage Tank	5,000 gallon circular tank	2.5 days detention time at peak flow of 100,000 gallons per day

The projected peak flow to be treated by the existing components is 100,000 gallons per day. This flow can occur over a 16 hour time period resulting in an hourly flow of 6,250 gallons per hour, or 104 gallons per minute. Based on a peaking factor of two the existing wastewater pretreatment system has the capacity to pretreat projected flows and loadings upstream of the new biological treatment components that will have to be installed to meet BOD and nitrogen treatment levels. As a means of providing system reliability and redundancy the biological system will include a mixed anoxic pretreatment lagoon to provide additional BOD removal prior to the oxic/anoxic biological treatment process (cyclic reactor) that will remove BOD and nitrogen to levels acceptable for disposal by subsurface disposal.

The existing system has the following limitations that must be addressed to handle the current and future processing plant wastewater flows:

 The physical/chemical pretreatment system is not capable of meeting the BOD and Nitrogen limits required by TDEC for management in a subsurface disposal system. A biological treatment system designed to

- nitrify and denitrify will be required to adequately treat processing plant wastewater upstream of subsurface disposal.
- The existing subsurface disposal system does not have the hydraulic capacity required to manage the projected processing plant treated wastewater flow. Hydraulic loading rates to the existing subsurface disposal areas (LPP and Chamber), if used, will have to be reduced to comply with nitrate nitrogen loading requirements.
- The existing subsurface disposal system has received extremely high hydraulic and organic loadings in the past and TDEC has expressed serious reservations regarding the acceptability of the existing subsurface areas to provide adequate treatment. Southeastern Provision intends to work with TDEC to assess the integrity of the existing subsurface areas with the hopes of possibly using the areas in the future to increase the overall hydraulic capacity of the entire system.

UPGRADED SYSTEM DESIGN BASIS

At peak production capacity the beef plant will process 400 head of cattle. Based on a reasonably liberal flow of 250 gallons per head per day the plant will generate approximately 100,000 gallons per day of wastewater to be handled.

Table No. 2 summarizes test data for typical samples taken from the discharge of the dissolved air flotation pretreatment system.

Table No. 2
Wastewater Characteristics (DAF Effluent)

Parameter	8/2/2018	8/15/2018
pH	7.4	5.5
Alkalinity, mg/l	550	151
COD, mg/l	782	
BOD5, mg/l	486	402
TSS, mg/l	75.8	18.1
TKN, mg/l	85	54.3
NH3N, mg/i	110	62.2
Total Phosphorus, mg/l	0.109	0.0928

In order to provide for a conservative design the following influent and effluent design parameters will be used for the design of the biological treatment system that will be installed upstream of the new subsurface disposal systems.

Parameter	Design Influent	Design Effluent
Flow, gpd	100,000	100,000
BOD, mg/l	1,000	<20
TSS, mg/l	200	<10
Total Nitrogen, mg/l	150	<20
Total Kjeldahl Nitrogen, mg/l	150	<10
Total Nitrate/Nitrite Nitrogen, mg/	0	<10

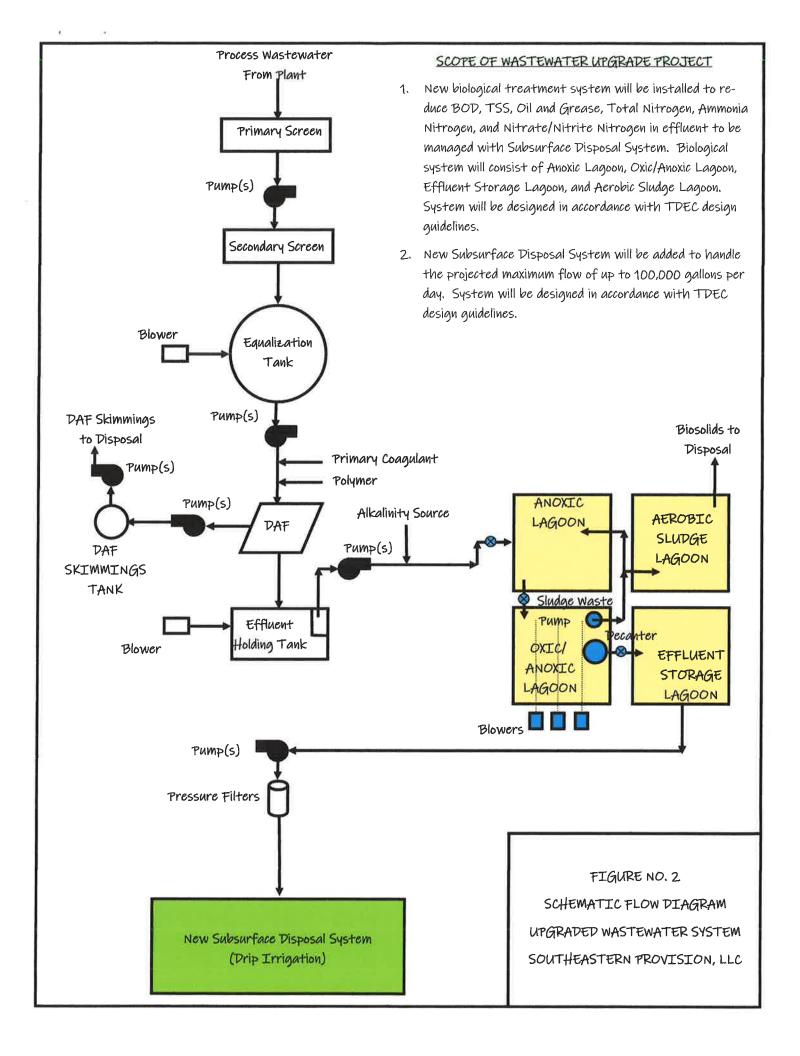
DESCRIPTION OF PROPOSED SYSTEM

Figure No. 2 provides a flow schematic for the proposed upgraded wastewater treatment system and subsurface disposal system. Figure No. 3 provides a preliminary layout drawing for the lined earthen basin biological treatment system that is proposed.

The proposed upgrades will include:

- DAF effluent transfer pumps to pump from the existing DAF effluent holding basins to the new biological treatment system
- New Membrane Lined Earthen Basin Biological Treatment System consisting of:
 - Lined Mixed Anoxic Lagoon for flow equalization, BOD pretreatment, organic nitrogen conversion to ammonia nitrogen, and denitrification
 - Lined Mixed Oxic/Anoxic Lagoon for BOD/TSS removal and nitrification/denitrification; lagoon will operate as a continuously fed intermittently decanted reactor basin (Cyclic Reactor)
 - o Oxic/Anoxic Lagoon Diffused Aeration System with:
 - Three Positive Displacement (PD) Blowers
 - Diffused Aeration System with floating laterals and retrievable fine bubble diffusers
 - o Floating Decanters for Oxic/Anoxic Lagoon
 - Lined Effluent Storage Lagoon
 - Sludge Wasting and Recycle Pumps for Oxic/Anoxic Lagoon
 - Lined Aerobic Sludge Digestion/Storage Lagoon with Supernent Return Pumps
- New Subsurface Disposal System Dosing Pumps
- Pressure filters
- Enlarged SSDS consisting of drip dispersion. The existing LPP and chamber areas may be used in the future if work with TDEC indicates the integrity of the areas is acceptable.

Table No. 3 provides a design analysis for the biological treatment system. Table No. 4 summarizes the details for the upgraded treatment system.



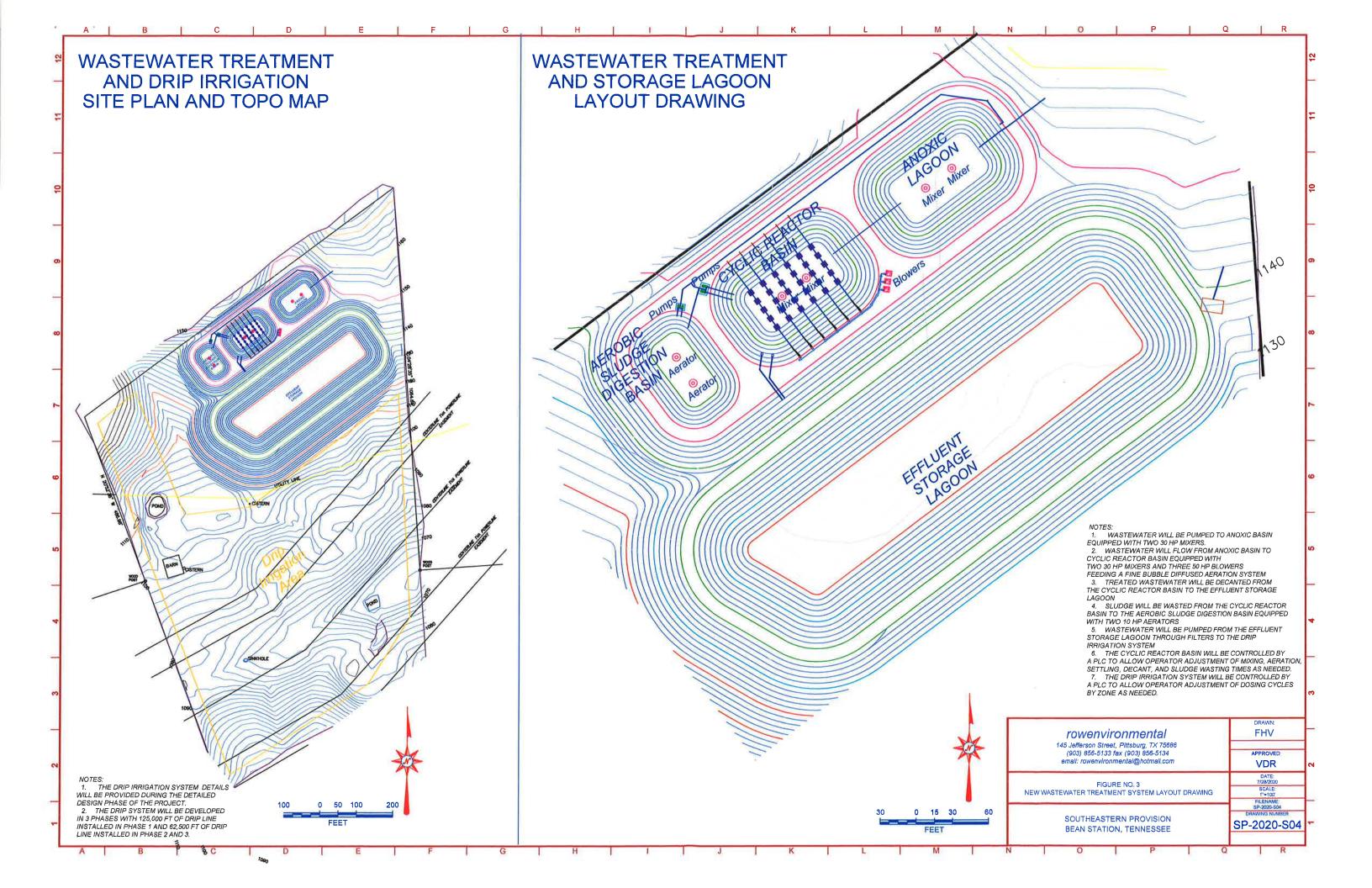


Table No. 3 Design Analysis

Cyclic Reactor Process - 0.1 mgd capacity LINED Earthen Basin with Diffused Aeration Project:

Southeastern Provision, Bean Station, TN

Project Assumptions:

- 1. Average wastewater characteristics to be below levels listed below.
- 2. Sufficient alkalinity exists in wastewater for nitrification. Supplemental alkalinity to be provided by Owner if needed.
- 3. No heavy metals or toxic organic compounds are present in the wastewater which will inhibit biological treatment.

Loadings/Effluent Requirements:

Loadings	DAF Effluent/ Anaerobic Lagoon Inf.	Anaerobic Lagoon Eff./ Oxic/Anoxic Inf.	Oxic/ Anoxic Effluent
Flow, mgd	0.1	0.1	0.1
BOD, mg/l	1,000	600	20
BOD, lbs/day	834	500	17
TSS, mg/l	200	100	10
TSS, lbs/day	167	83	8
TKN, mg/l	150	150	10
TKN, lbs/day	125	125	8
NH3N, mg/l	100	150	10
NH3N, lbs/day	83	125	8
NO3N + NO2N, mg/l	0	0	10
NO3N + NO2N, lbs/day	0	0	8
TN, mg/l	150	150	20
TN, lbs/day	125	125	17

Southeastern Provision, Bean Station, TN

Design	Anaerobic	Comments
Loading, lbs/day	834	Loading, lbs/day = Flow, mgd x Influent BOD, mg/l x 8.34
Design Load, lbs BOD/day/1,000 cu ft		Normal value 5 to 20; use 6.5
Volume, gallons	960,383	Volume, gal = Volume, cu ft x 7.485
Detention Time, days	9.60	Detention Time, days = Volume, gal / Flow, gpd
Volume at High Level, cu ft	128,308	Volume = Loading x 1000 / Design Load
Average Surface Area, sq ft	9,165	Avg Surface Area = Volume, cu ft / Depth, ft
Width to Length Ratio	0.68	Design assumption
Side Slope	2.0	Design assumption
Depth at High Level, feet	14	Design assumption
Length at High Water Level, feet	147	(*) See equation below
Width at High Water Level, feet	100	Width = Length at High Water x Width to Length Ratio
Length at Bottom, feet	91	Length = Length at High Water - (Depth at High Level x Side Slope x 2)
Width at Bottom, feet	44	Width = Width at High Water Level - (Side Slope x Depth at High Level x 2)
Freeboard, feet	2	Design assumption
Total Depth, feet	16	Total Depth = Depth at High Level + Freeboard
Length at Inside Bank, feet	155	Length = Length at High Water + (Freeboard x Side Slope x 2)
Width at Inside Bank, feet	108	Width = Width at High Water Level + (Freeboard x Side Slope x 2)

 $(*) \ Length = ((((4 \times W:L \times Avg \ Area) - (6 \times W:L \times (Slope \times HWL)^2) + (2 \times (Slope \times LWL)^2))^0.5) + ((Slope \times HWL)^*(1+W:L)))/(2*W:L)$

Table No. 3 - Design Analysis (continued) Cyclic Reactor Process - 0.1 mgd capacity LINED Earthen Basin with Diffused Aeration Project: Southeastern Provision, Bean Station, TN

Design	Cyclic Reactor	Comments
Detention Time at Low Level, days	9.35	Detention Time = Low Volume / Flow
Detention Time at High Level, days	9.60	Detention Time = High Volume / Flow
BOD Loading, Ibs/day	500	BOD Loading, lbs/day = Flow, mgd x BOD, mg/l x 8.34
Food to Microorganism Ratio	0.029	Normal 0.01 to 0.1; use 0.22
MLVSS in System, Ibs	17,553	MLVSS, lbs/day = BOD Loading / F:M Ratio
Volatile Solids, percent	75.00	Design assumption
MLSS in System, Ibs	23,403	MLSS, lbs/day = MLVSS / (65% Volatile Solids/100)
MLSS at Low Level, mg/l	3,000	Design assumption
Volume at Low Level, gallons	935,390	Volume, gal = MLSS, lbs / (MLSS, mg/l x 8.34)
Cycles per day	4	Design assumption
Volume Decanted each Cycle, gallons	25,000	Cycles = Flow / Cycles per day
Excess Decant Volume Capacity, percent	0	Design assumption
Volume at High Level, gallons	960,390	High Volume = Low Vol + Decant Vol + Excess Decant Vol
Volume at High Level, cu ft	128,309	Volume, cu ft = Volume, gal / 7.485
Average Surface Area, sq ft	9,165	Avg Surface Area = Volume, cu ft / Depth, ft
Width to Length Ratio	0.68	Design assumption
Side Slope	2.0	Design assumption
Depth at High Level, feet	14	Design assumption
Length at High Water Level, feet	147	(*) See equation below
Width at High Water Level, feet	100	Width = Length at High Water x Width to Length Ratio
Length at Bottom, feet	91	Length = Length at High Water - (Depth at High Level x Side Slope x 2)
Width at Bottom, feet	44	Width = Width at High Water Level - (Side Slope x Depth at High Level x 2)
Freeboard, feet	2	Design assumption
Total Depth, feet	16	Total Depth = Depth at High Level + Freeboard
Length at Inside Bank, feet	155	Length = Length at High Water + (Freeboard x Side Slope x 2)
Width at Inside Bank, feet	108	Width = Width at High Water Level + (Freeboard x Side Slope x 2)

^(*) Length = $((((4 \times W:L \times Avg Area)-(6 \times W:L \times (Slope \times HWL)^2)+(2 \times (Slope \times LWL)^2))^0.5)+((Slope \times HWL)^*(1+W:L)))/(2^*W:L)$

Southeastern Provision, Bean Station, TN

Design	Cyclic Reactor	Comments
Depth at Low Level, feet	11.75	
BOD Removal Efficiency, %	96.67%	BOD Rem Eff = 100 x (Inf BOD - Eff BOD) / Inf BOD
BOD Removed, Ibs/day	484	BOD Removed = BOD Applied x BOD Rem Eff
TKN Removal Efficiency, %	93.33%	TKN Rem Eff = 100 x (Inf TKN - Eff TKN) / Inf TKN
TKN Removed, lbs/day	117	TKN Removed = TKN Applied x TKN Rem Eff
Oxygen Requirement, Ibs/Ib BOD Rem	1.5	Normal Range = 1 to 1.5; use 1.5 for cyclic process
Oxygen Requirement, lbs/lb NH3N Rem	4.6	Normal Requirement = 4.6
BOD AOR, lbs/day	726	BOD AOR = BOD Removed x BOD Oxygen Reqd
TKN AOR, Ibs/day	537	TKN AOR = TKN Removed x TKN Oxygen Reqd
Denitrification Credit	40.00%	Normal Value = 50 %; use 40 %
Total AOR, lbs/day	1,048	Total AOR = BOD AOR + TKN AOR - Denitrification Credit
Temperature, deg C	28	Design assumption
Elevation, feet	1150	Given
Average Water Depth in Reactor, ft	12.9	Design assumption
Alpha	0.7	Normal Range = 0.7 to 0.85; use 0.7
Beta	0.8	Normal Range = 0.8 to 1.0; use 0.8
Theta	1.024	Constant
Operating DO, mg/l	2	Design assumption
C-SC, mg/l (constant)	9.09	Saturation DO at standard conditions
C-DC, mg/l (saturation at site conditions)	7.81	Saturation DO at site conditions
Oxygen Transfer Adjustment Factor		Factor = (((C-DC x Beta)-Op DO) x (Theta^(Temp -20)) x Alpha)/C-SC
SOR, lbs/day		SOR = AOR / Oxygen Transfer Adjustment Factor
SOR, lbs/hr		SOR, lbs/hr = SOR, lbs/day / 24
Time of Aeration during each Cycle, hours	3.00	Design assumption

Southeastern Provision, Bean Station, TN

Basis

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Total Aeration, hours per day SOR During Aeration, lbs/hr Aeration Type Oxygen Transfer, lbs SOR/hp-hr (clean water) Theoretical Brake Horsepower for Aeration, hp Installed Aeration Brake Horsepower, hp

12.0 Total Aeration, hrs = Cycles per day x Time of Aeration During Each Cycle 221 SOR During Aeration, lbs/hr = SOR, lbs/hr x (24 / Total Aeration, hours/day) Diffused Aeration Design selection 4 From equipment supplier 55.2 BHP = SOR During Aeration / Oxygen Transfer 150 Use 3 @ 50

Design

Decant Basin Volume, gallons Decant Basin Volume, cu ft Average Surface Area, sq ft Width to Length Ratio Side Slope Depth at High Level, feet Length at High Water Level, feet Width at High Water Level, feet Length at Bottom, feet Width at Bottom, feet Freeboard, feet Total Depth, feet Length at Inside Bank, feet

Width at Inside Bank, feet

Storage Basin Basis

Cyclic Reactor

7,700,000 Volume = Design Flow, apd 1,028,724 Volume, cu ft = Volume, gal / 7.485 57,151 Avg Surface Area = Volume, cu ft / Depth, ft 0.34 Design assumption 2.5 Design assumption 18 Design assumption 498 (*) See equation below 169 Width = Length at High Water x Width to Length Ratio 408 Length = Length at High Water - (Depth at High Level x Side Slope x 2) 79 Width = Width at High Water Level - (Side Slope x Depth at High Level x 2) 2 Design assumption 20 Total Depth = Depth at High Level + Freeboard 508 Length = Length at High Water + (Freeboard x Side Slope x 2) 179 Width = Width at High Water Level + (Freeboard x Side Slope x 2)

(*) Length = ((((4 x W:L x Avg Area)-(6 x W:L x (Slope x HWL)^2)+(2 x (Slope x LWL)^2))^0.5)+((Slope x HWL)^*(1+W:L)))/(2*W:L)

Southeastern Provision, Bean Station, TN

Sludge Analysis Si	ludge Basin	Comments
Sludge Production, lbs/lb BOD Rem	0.50	Normal Range = 0.3 to 1.0; use 0.50
Sludge Production, lbs/lb TKN Rem	0.15	Normal Range = 0.1 to 0.2; use 0.15
Total Sludge Production, Ibs/day	346	Total Sludge Production = (BOD Removed x 0.5 + TKN Removed x 0.15) / % Volatile Solids
Sludge Age, days	67.7	Sludge Age, days = Total Sludge in System, lbs / Total Sludge Production, lbs/day
Sludge Solids Content, %	1	Design assumption
Sludge Volume, gallons/day	4,147	Sludge Volume, gpd = (Sludge Production, lbs/day / (Sludge Solids, % / 100)) x 8.34
Sludge Wasting Each Cycle, minutes	30	Design assumption
Sludge Wasted Each Cycle, gallons	1,037	Sludge, gal/cycle = Sludge, gal/day / Cycles per day
Sludge Pump Flow Rate, gpm	35	Pump Rate = Gal per cycle / Wasting time per cycle, min; use 2 @ 50 gpm
Sludge Basin Detention Time, days	65.0	Design assumption
Sludge Basin Volume, cubic feet	36,010	Volume, cu ft = Volume, gal / 7.485
Sludge Basin Volume, gallons	269,533	Volume, gal = Detention Time, days x Sludge volume, gal/day
Average Surface Area, sq ft	4,501	Avg Surface Area = Volume, cu ft / Depth, ft
Width to Length Ratio	0.65	Design assumption
Side Slope	2.0	Design assumption
Depth at High Level, feet	8	Design assumption
Length at High Water Level, feet	102	(*) See equation below
Width at High Water Level, feet	66	Width = Length at High Water x Width to Length Ratio
Length at Bottom, feet	70	Length = Length at High Water - (Depth at High Level x Side Slope x 2)
Width at Bottom, feet	34	Width = Width at High Water Level - (Side Slope x Depth at High Level x 2)
Freeboard, feet	2	Design assumption
Total Depth, feet	10	Total Depth = Depth at High Level + Freeboard
Length at Inside Bank, feet	110	Length = Length at High Water + (Freeboard x Side Slope x 2)
Width at Inside Bank, feet	74	Width = Width at High Water Level + (Freeboard x Side Slope x 2)

(*) Length = $((((4 \times W:L \times Avg Area)-(6 \times W:L \times (Slope \times HWL)^2)+(2 \times (Slope \times LWL)^2))^0.5)+((Slope \times HWL)^*(1+W:L)))/(2*W:L)$

Southeastern Provision, Bean Station, TN

Sludge

	Oldage	
Sludge Analysis	Basin	Comments
Total Sludge Production, lbs/day	346	Total Sludge Production = (BOD Removed x 0.4 + TKN Removed x 0.15) / % Volatile Solids
Biomass in Sludge, Ibs/day	259	Biomass = Total Sludge Production x % Volatile Solids
Biomass Reduced in Digester, %	50	Design Assumption
Biomass Reduced, Ibs/day	130	Biomass Reduced = Biomass in Sludge, lbs/day x Biomass Reduced, %
Oxygen Requirements, lb/lb Solids	2.3	Design assumption
Oxygen Requirements, Ibs/day (standard conditions)	298	Oxygen Reg'd = Biomass Reduced, lbs/day x O2 Reg'd, lbs/lb solids
Oxygen Requirements, Ibs/day (site conditions)	597	Air, site conditions = Air, standard conditions/Oxygen Transfer Adj. Fact.
Oxygen Transfer Rate, lbs/ hp hr	3	Design assumption
Theoretical Aerator HP	8	Aerator HP = Oxygen Reqd, lbs/day/24/Oxygen Transfer Rate
Installed Aerator HP	20	(USE 2 @ 10 HP)

Table No. 4 Component Details Upgraded Wastewater Management System

Component	Description
Pretreated Wastewater Transfer Pumps	Two self priming centrifugal pumps with VFD's; 250 gpm
Lined Anoxic Lagoon	960,383 gallons; 108 ft wide by 155 ft long by 16 ft deep; lined with 80 mil HDPE liner; two 30 hp floating mixers
Lined Oxic/Anoxic Lagoon	960,383 gallons; 108 ft wide by 155 ft long by 16 ft deep; lined with 80 mil HDPE liner; two 30 hp floating mixers; three 50 hp blowers with diffused aeration; two floating decanters
Lined Effluent Storage Lagoon	7,700,000 gallons; 179 ft wide by 508 ft long by 20 ft deep; lined with 80 mil HDPE liner
Lined Aerobic Sludge Lagoon	269,533 gallons; 74 ft wide by 110 ft long by 10 ft deep; lined with 80 mil HDPE liner; two 10 hp floating aerators
Oxic/Anoxic Lagoon Aeration System	Three 50 hp positive displacement blowers with VFD's; floating diffuser laterals with retrievable fine bubble diffusers
Oxic/Anoxic Lagoon Decant System	Two 600 gpm floating decanters
Sludge Wasting and Recycle Pumps	Two self priming centrifugal pumps with VFD's; 100 gpm
New Subsurface Disposal System Dosing Pumps	Two new centrifugal pumps will be provided for the new drip dispersal areas; new pumps will be 250 gpm with VFD's
Pressure Filters	New pressure multimedia filtration system will be installed upstream of drip system; new filters will have capacity of 250 gpm
Subsurface Disposal System	The drip dispersal system will be required to handle up to 100,000 gallons per day at peak flow if the existing subsurface disposal areas can not be used in the future. A total of up to 500,000 sf of drip area will be required. This will require up to 250,000 feet of drip tubing. See Table No. 5. The drip system will be constructed in phases with 250,000 sf installed initially followed by up to 125,000 sf in each of two more phases.

Southeastern Provision has adequate land to handle the projected peak design flow of 100,000 gallons per day plus a 100 percent reserve area. The facility is currently generating approximately 40,000 gallons per day of wastewater. The peak flow of 100,000 gallons per day will not be reached for many years depending upon market conditions. The construction of the wastewater treatment and subsurface disposal upgrades will be staged as follows within the allowable special conditions of the permit:

- Phase 1 The biological wastewater treatment and storage system will be constructed in conjunction with the addition of 250,000 sf of drip area.
 The wastewater system will be constructed initially to handle the peak flow of 100,000 gallons per day. This will allow for a very conservative design during startup and allow the facility to demonstrate design performance.
- Phase 2 Addition of 125,000 sf of drip area
- Phase 3 Addition of 125,000 sf of drip area

Table No. 4

Hydraulic and Nutrient Loading Calculations

Subsurface Disposal System

Southeastern Provision, Bean Station, TN

Lwn =		Calculated Allowable Nitrate Loading Rate	
Pr =		5-yr return monthly precipitation (in/month)	Subsurface Disposal Systems will be
PET =		Potential evapotransporation (in/month)	designed based on hydraulic loading
N =	Uptake	Monthly nitrogen uptake rate by vegetation (lbs/acre/month)	of 0.2 gpd/sf. The loading is below the
F =		Applied nitrogen fraction removed by denitrification/volatization (%(allowable nitrogen based loading rate.
Cp =	10	Maximum nitrate concentration in leachate (mg/l)	
Cn =	20	Nitrogen concentration in applied wastewater (mg/l)	Depth to water table in the SSDS areas
	4.413	Conversion factor	is greater than 200 cm.
U =	100	Annual nitrogen uptake rate for crop, variable (lbs/acre/year)	
Ksat = 9.17 Saturated hydraulic conductivity (micrometers/sec) (NRCS)		Saturated hydraulic conductivity (micrometers/sec) (NRCS)	Depth to most restrictive zone in the
Ksat = 31.19 Saturated hydraulic conductivity (inches/day) (NRCS)		Saturated hydraulic conductivity (inches/day) (NRCS)	SSDS areas is greater than 200 cm.
Lwh (max) =	5.00	Applied effluent max loading rate (gpd/sf) (10% NRCS Ksat)	
Lwh (max) =	0.25	Applied effluent max loading rate (gpd/sf) (TDEC max)	

MONTH	Pr, in/month	PET, in/month	N Uptake, %/month	N Uptake, lb/ac/ month	F (Denitrification), %/month	Lwn, in/month	Lwn, in/week	Lwn, in/day	Lwn, gpd/sf	Lwh, gpd/sf (based on Loam soil)
Jan	7.62	0.1	1%	1	25%	15.92	3.60	0.51	0.320	0.20
Feb	6.72	0.27	2%	2	25%	14.67	3.67	0.52	0.327	0.20
Mar	8.85	0.97	4%	4	27%	20.97	4.73	0.68	0.422	0.20
Apr	6.59	2.3	8%	8	29%	18.62	4.34	0.62	0.387	0.20
May	6.13	3.59	12%	12	31%	20.62	4.66	0.67	0.415	0.20
Jun	5.52	4.9	15%	15	33%	21.29	4.97	0.71	0.443	0.20
Jul	6.85	5.44	17%	17	35%	29.71	6.71	0.96	0.598	0.20
Aug	4.73	5	15%	15	35%	21.17	4.78	0.68	0.426	0.20
Sep	5.54	3.79	12%	12	34%	22.02	5.14	0.73	0.458	0.20
Oct	4.47	1.98	8%	8	32%	16.72	3.78	0.54	0.336	0.20
Nov	6.11	0.82	4%	4	29%	16.80	3.92	0.56	0.349	0.20
Dec	7.55	0.27	2%	2	26%	17.01	3.84	0.55	0.342	0.20
Total	76.68	29.43	100%	100		235.50462				

As has been noted, the facility currently has 15,000 ft of existing subsurface disposal area that is being restricted from use by TDEC due to potentially excessive hydraulic and organic loading in the past. Southeaster Provision intends to work with TDEC to determine if any or all of the existing fields can be brought back on line a some point in the future. It the existing subsurface systems can be utilized the drip areas installed in Phase 2 and Phase 3 may vary.

SOLIDS HANDLING AND DISPOSAL

The facility generates or will generate the following residual solids that require handling and disposal:

- Manure
- DAF Skimmings
- Biosolids

An application for a permit by rule to allow the land application of these materials has been submitted to TDEC under separate cover.

DOMESTIC POTABLE WELLS WITHIN 1000 FEET OF FACILITY

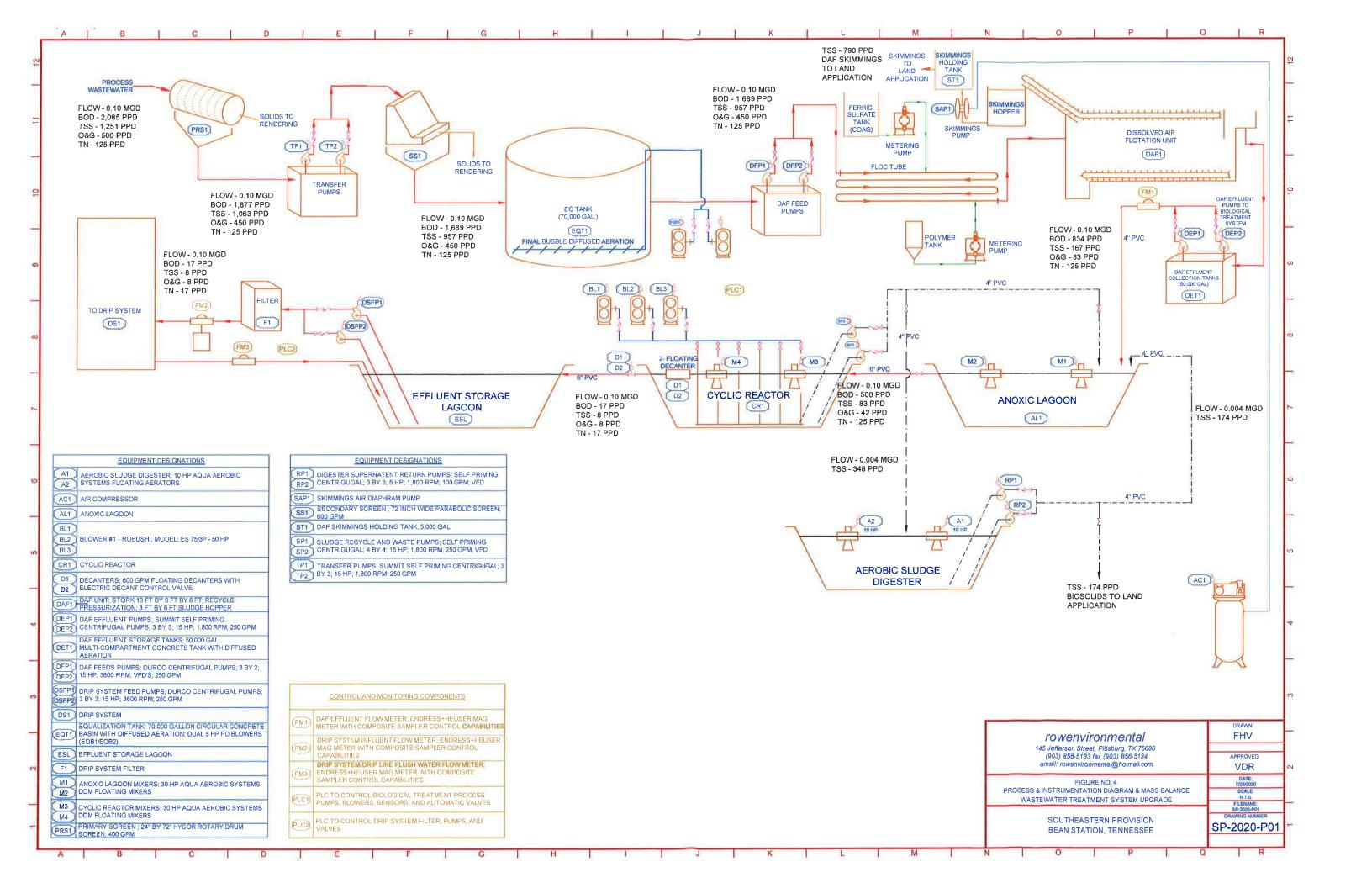
The attached topographic map shows known potable wells within 1000 feet of the plant.

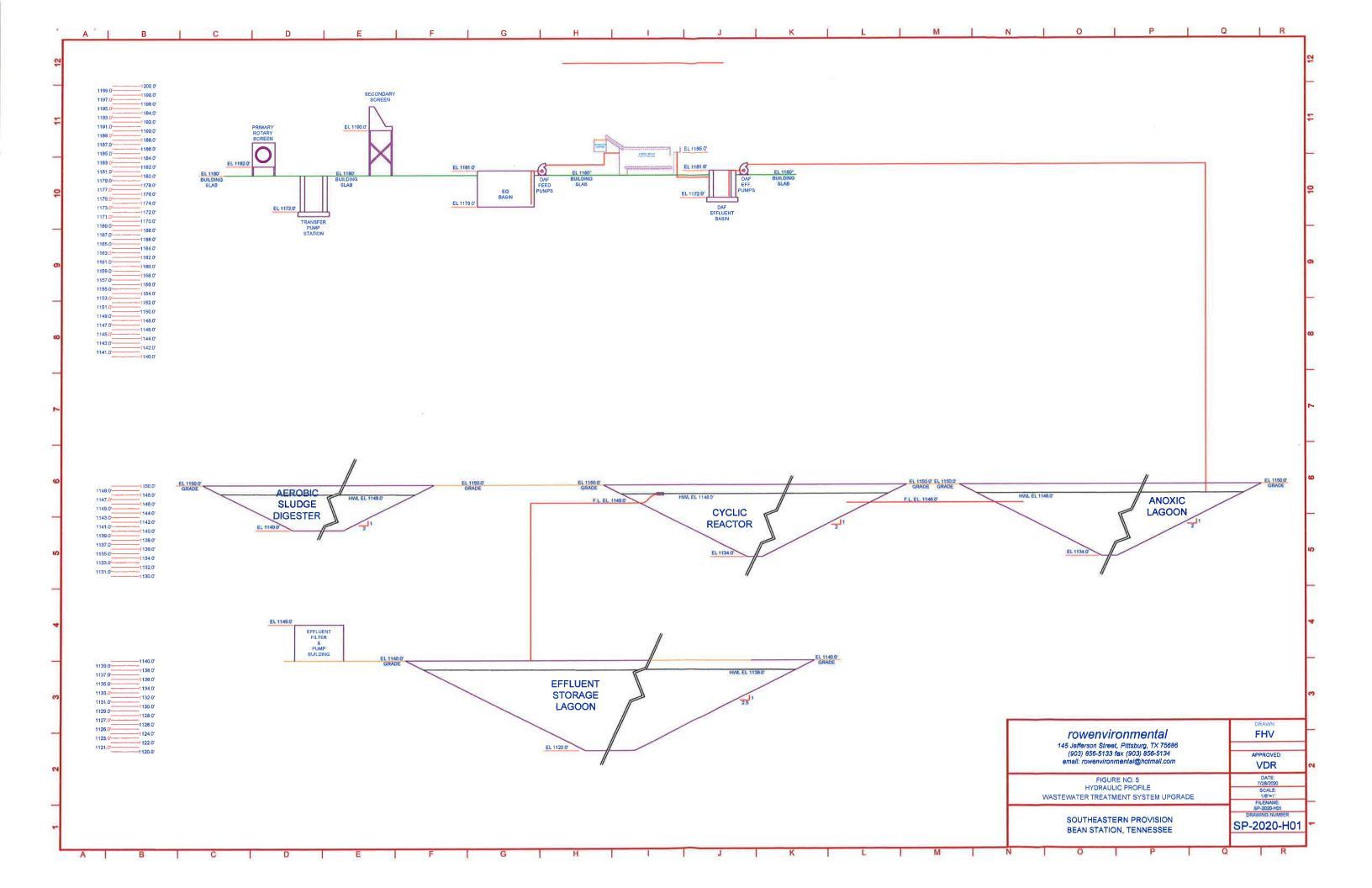
MASS BALANCE

Figure No. 4 provides a process flow diagram with mass balance data for key parameters noted.

HYDRAULIC PROFILE

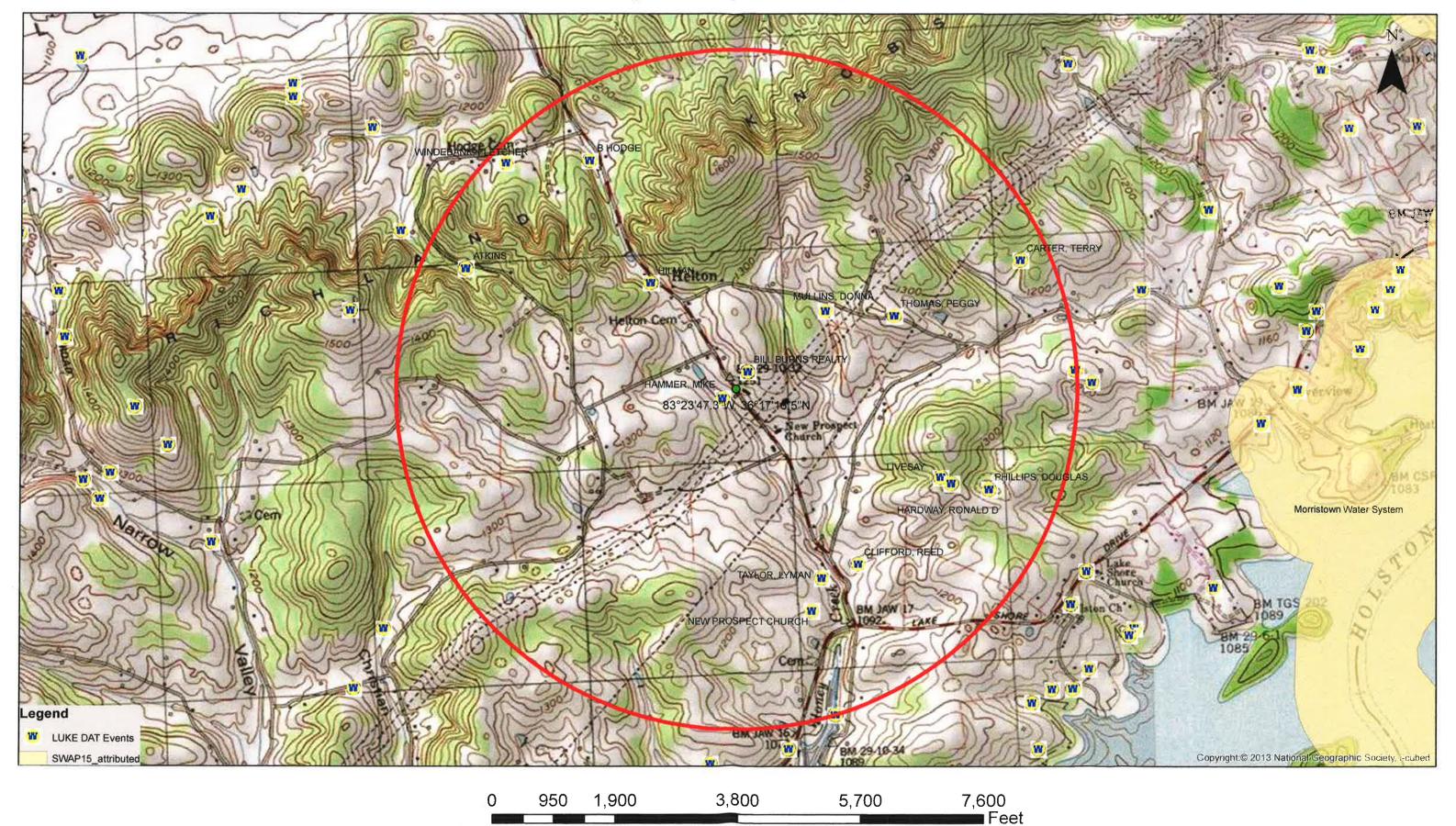
Figure No. 5 provides a hydraulic profile for the total system from the existing DAF pretreatment area to the new biological treatment system and drip subsurface dispersal system.





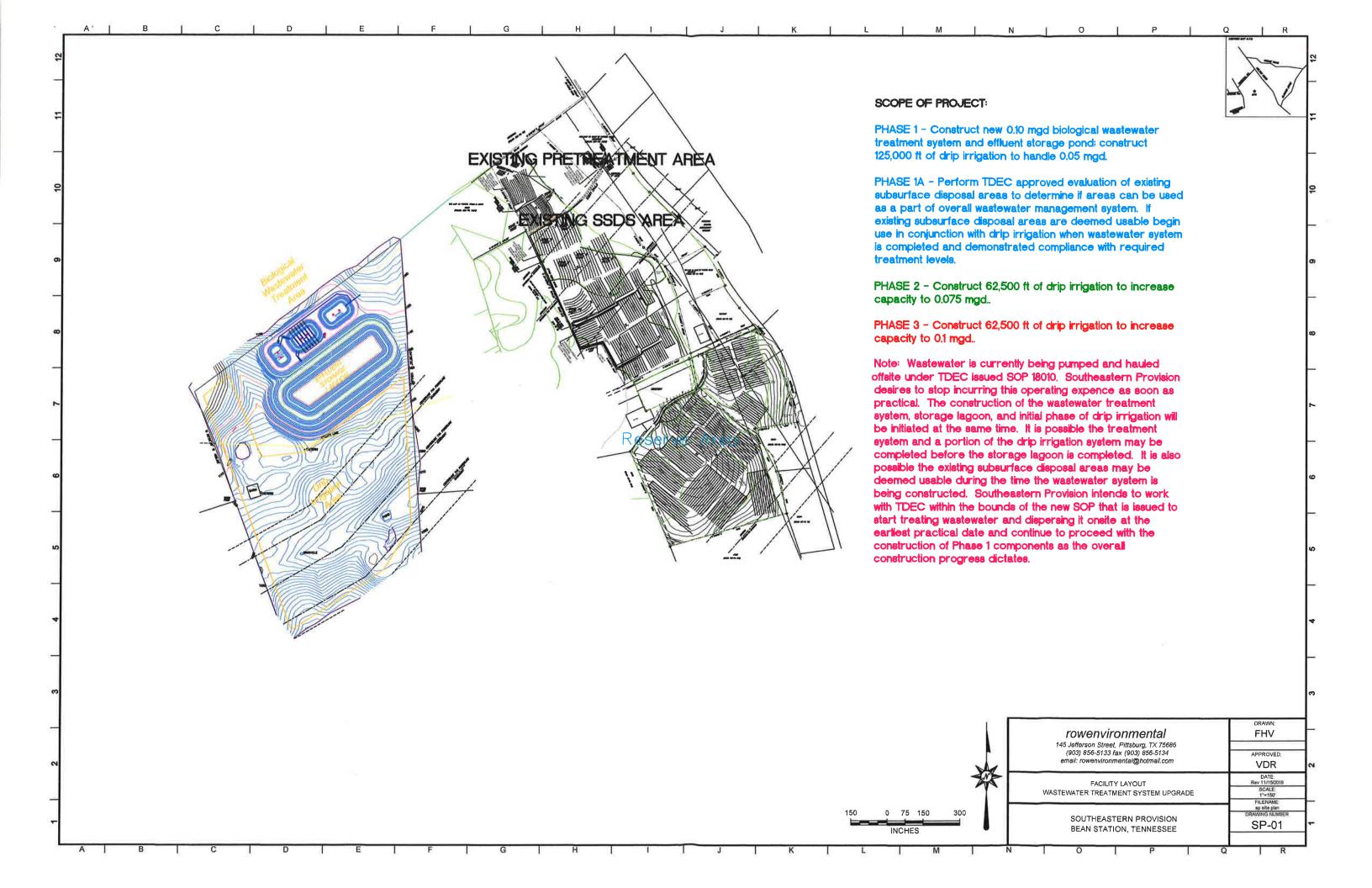
TOPOGRAPHIC MAP

Southeastern Provision



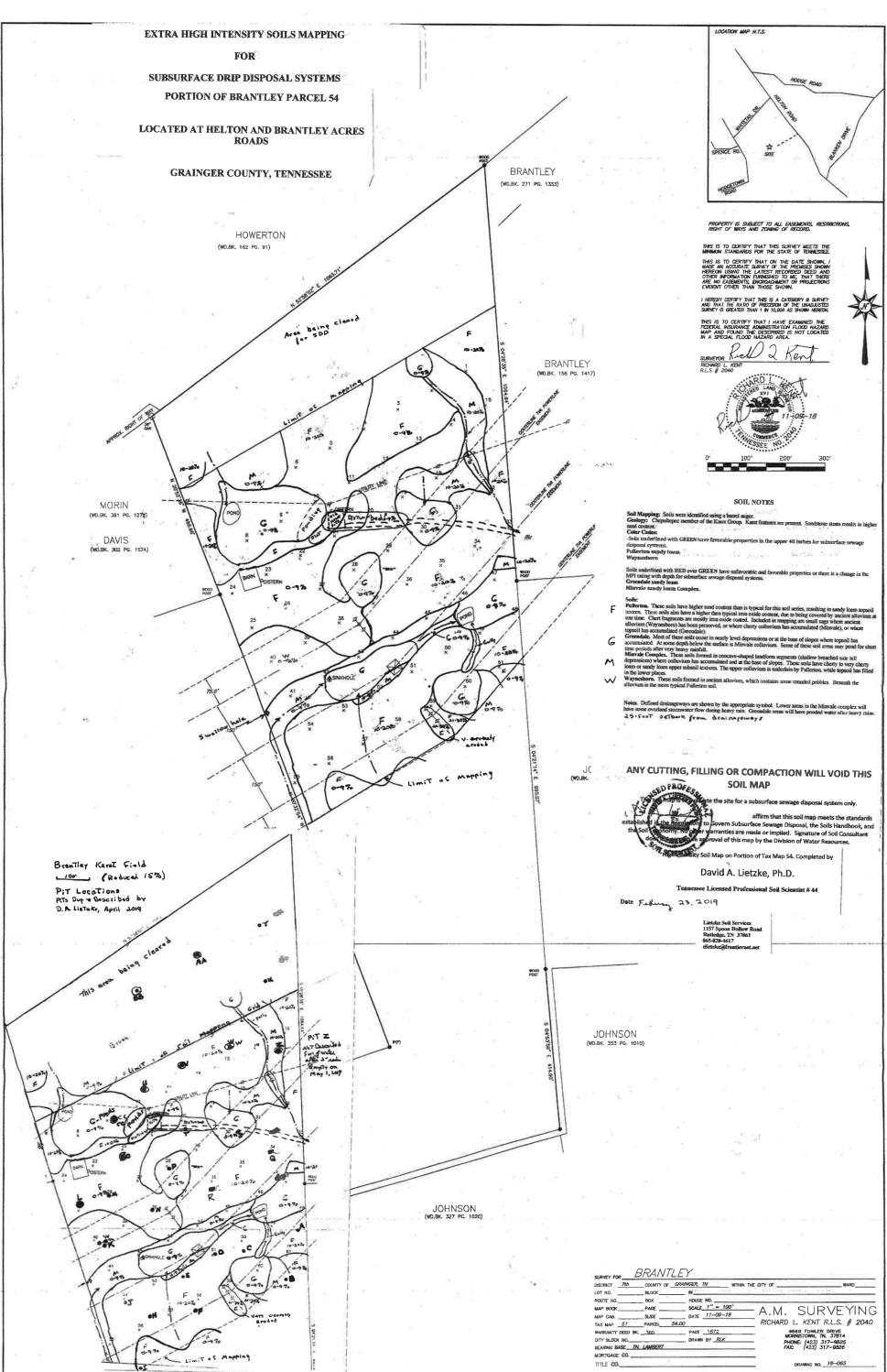
SCALED LAYOUT OF FACILITY

The attached layout drawing is a compilation of the site plan for the existing system superimposed onto an aerial that shows the proposed wastewater treatment area and proposed drip dispersal system area.



SSDS SOILS INFORMATION

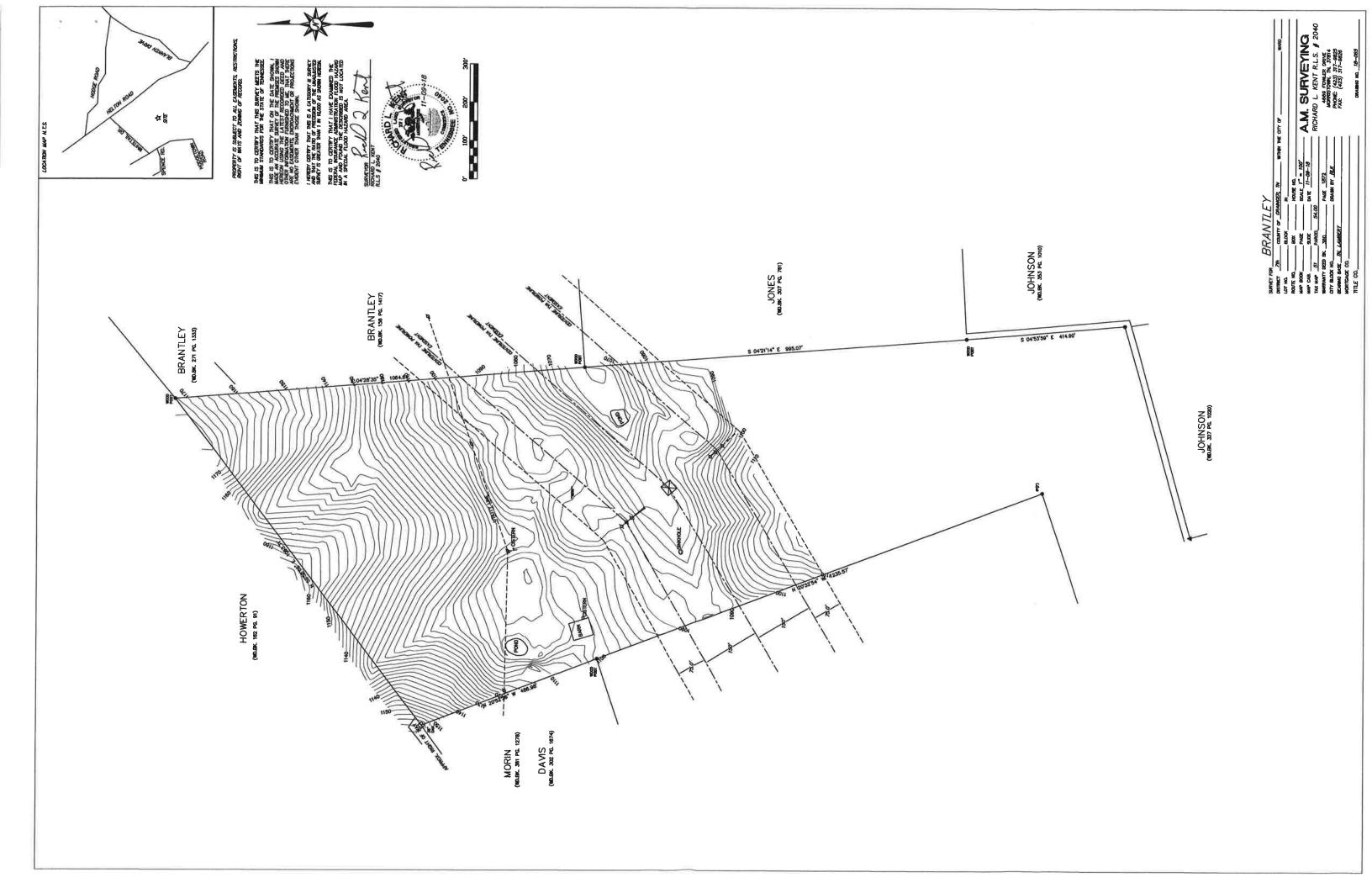
A soils map for the new proposed SSDS area is attached.



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TOPOGRAPHIC MAP OF WASTEWATER TREATMENT AND SUBSURFACE DISPOSAL SYSTEM

A topographic map for the proposed new wastewater treatment and SSDS area is attached.



DESCRIPTION OF ALTERNATIVE APPLICATION METHODS

There is no municipal/public sewer system than can feasibly be connected to.

Surface land application of all wastewater is not feasible due to site topography and the requirements for wet weather storage. After the biological treatment system and initial drip dispersal system are completed Southeastern Provision intends to evaluate the feasibility of surface land application to reduce the hydraulic loadings to the drip dispersal areas. If it is determined surface application of a part of the treated wastewater is technically and cost effectively feasible the appropriate permit amendment process will be pursued.

The only feasible option for the facility at this time is to treat the wastewater to the required level and dispose of the treated wastewater through a subsurface disposal system consisting of a new drip dispersal system and the potential future use of the existing subsurface fields if they are determined to be acceptable for use.

DRIP DISPERSAL ADDITIONAL INFORMATION

GROUNDWATER

Historical usage of groundwater in the area has primarily been for residential and agricultural purposes. Groundwater generally flows south to southwest.

POPULATION AND CULTURAL DEVELOPMENT IN AOR

The area is primarily an agricultural setting. There are approximately 15 residential homes within the area. Land use is primarily cattle farming. Cherokee Lake is within a one mile radius.

NATURE OF INJECTED FLUID

The fluid to be managed in the subsurface disposal system will be treated wastewater from a beef processing facility. The wastewater will be treated using a physical/chemical pretreatment system followed by a biological treatment system to produce an advance treated effluent including nitrogen removal. The biologically treated wastewater will be filtered prior to being pumped to the subsurface disposal system(s). Treated wastewater will have the following characteristics:

- pH 6 to 8
- BOD less than 20 mg/l
- Total Suspended Solids less than 10 mg/l
- Total Nitrogen less than 20 mg/l
- Oil and Grease less than 10 mg/l

TOPOGRAPHIC MAP SHOWING GROUNDWATER USAGE

A topo map with known well locations is attached.

WELL HEAD PROTECTION OR SOURCE WATER PROTECTION

The AOR is not located in a known well head protection or source water protection area. The topo maps shows the City of Morrisville, TN water system which is out side the AOR.

DESCRIPTION OF SYSTEM

The wastewater management system will consist of a physical/chemical pretreatment system followed by a biological treatment system. Biologically treated wastewater will be filtered and then pumped to the subsurface disposal system which will consist of drip dispersal with the possible future use of existing low pressure pipe and chamber areas in the future if determined to be effective. Wastewater flows at the time of preparation of this permit application are

approximately 40,000 gallons per day. Flows will increase to up to 100,000 gallons per day over a period of time depending on market conditions and the number of cattle processed.

NATURE AND TYPE OF SYSTEM

The biological treatment system will be a multi-basin oxic/anoxic (Cyclic Reactor) system designed to provide significant reliability and redundancy. The biological treatment system is being designed assuming limited treatment from the dissolved air flotation pretreatment system in order to provide a very conservative design as relates to hydraulic detention time, sludge age, and aeration requirements. All pumps and blowers will be designed with full standby units. The subsurface disposal system will be designed in strict accordance with TDEC regulations as relates to materials of construction. Multiple subsurface disposal areas will be provided. This will allow flexibility in how the individual areas are operated and rested . 100 percent reserve areas will be available.